

Detailed Intersection Modelling

Based on Analysis of the Interaction of

Conflicting Traffic Movements

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September 2011



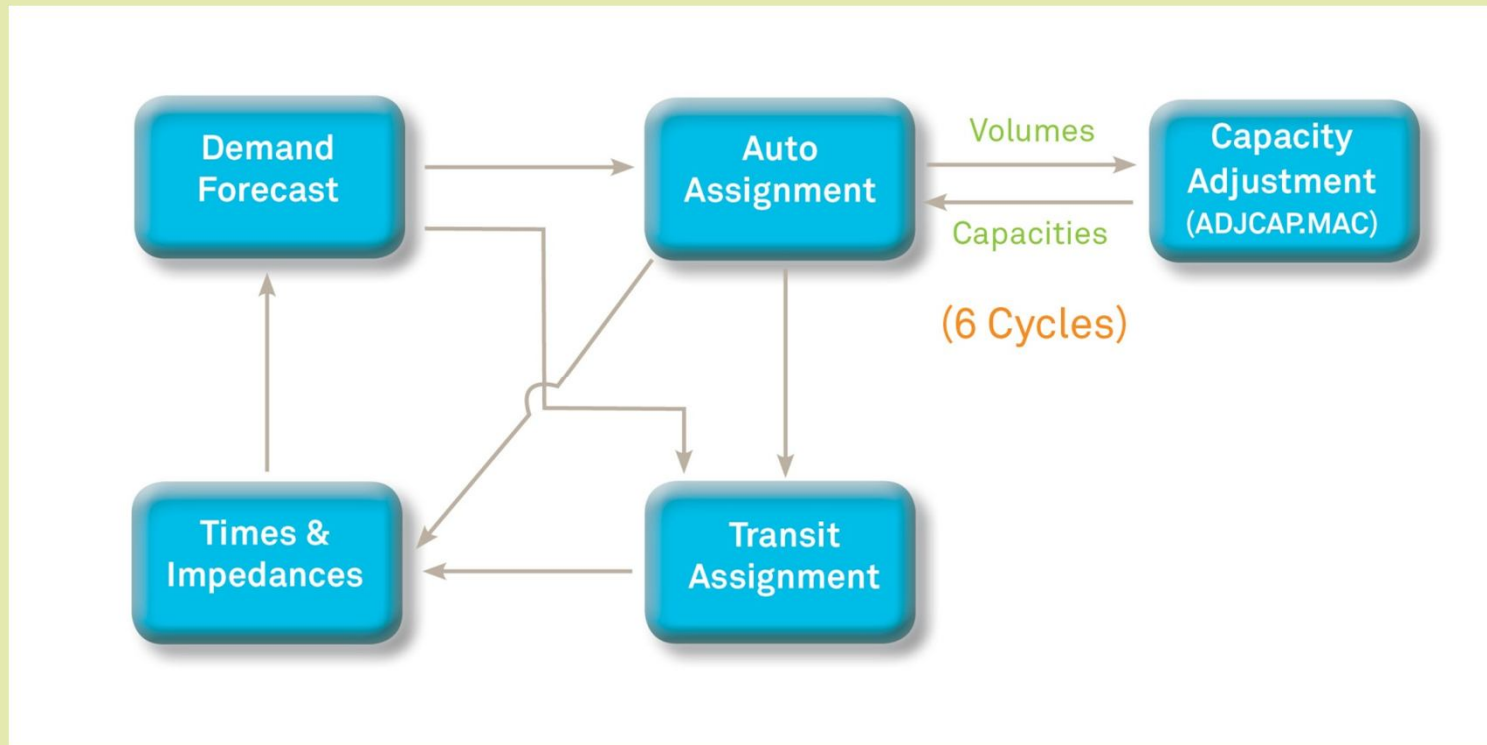
Intersection Modelling

- Used in Other Studies
- Primarily for Signalised Intersections
- Based on Signal Timing and Saturation Flows
- Impact of Conflicting Flows Generalised
- Fixed Turn Capacities
- Impressive Results

AECOM Approach

- Inclusion of Unsignalised Intersections
- Explicit Modelling of Conflicting Traffic
- Explicit Modelling of Capacities in Shared Lanes
- Iterative Procedure to Adjust Affected Capacities
- Procedures Based on HCM2010

Iterative Procedure



Signalised Intersections

Signalised Intersections

- Fixed Capacities for Protected Movements

$$c = s \frac{g}{C} N$$

- Variable Capacities
 - Permitted (and protected/permitted) LTs
 - Through and turn movements in shared lanes

Signalised Intersections

- Permitted Left Turns

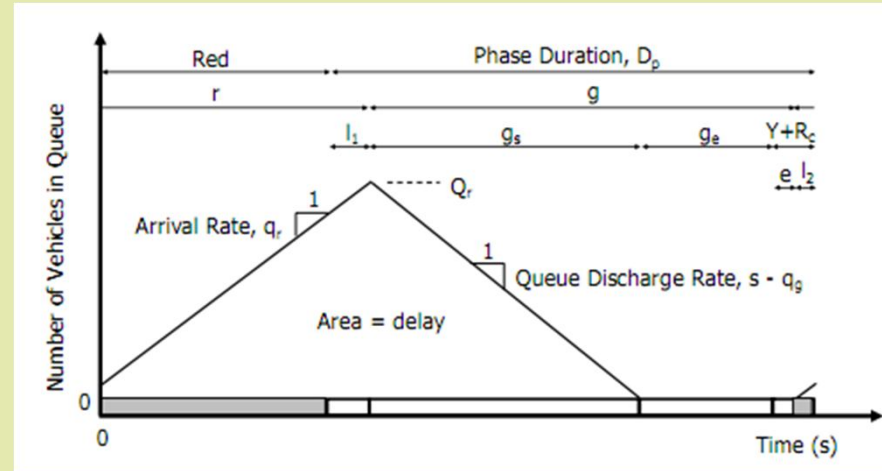
- HCM 2010 Saturation Flow Rate Equation
- Critical Headway 4.5 secs: Follow-up Headway 2.5 secs.

$$c_{pm} = \left(\frac{v_o e^{-4.5v_o/3600}}{1 - e^{-2.5v_o/3600}} \frac{g_u}{C} + \frac{2*3600}{C} \right) N$$

- v_o = opposing thru and RT volume from previous assignment
- C = cycle length
- N = number of permitted LT lanes
- g_u = green time (s) after clearance of initial queue of opposing traffic

Signalised Intersections

- Permitted Left Turns
 - Post-Queue Clearance Time

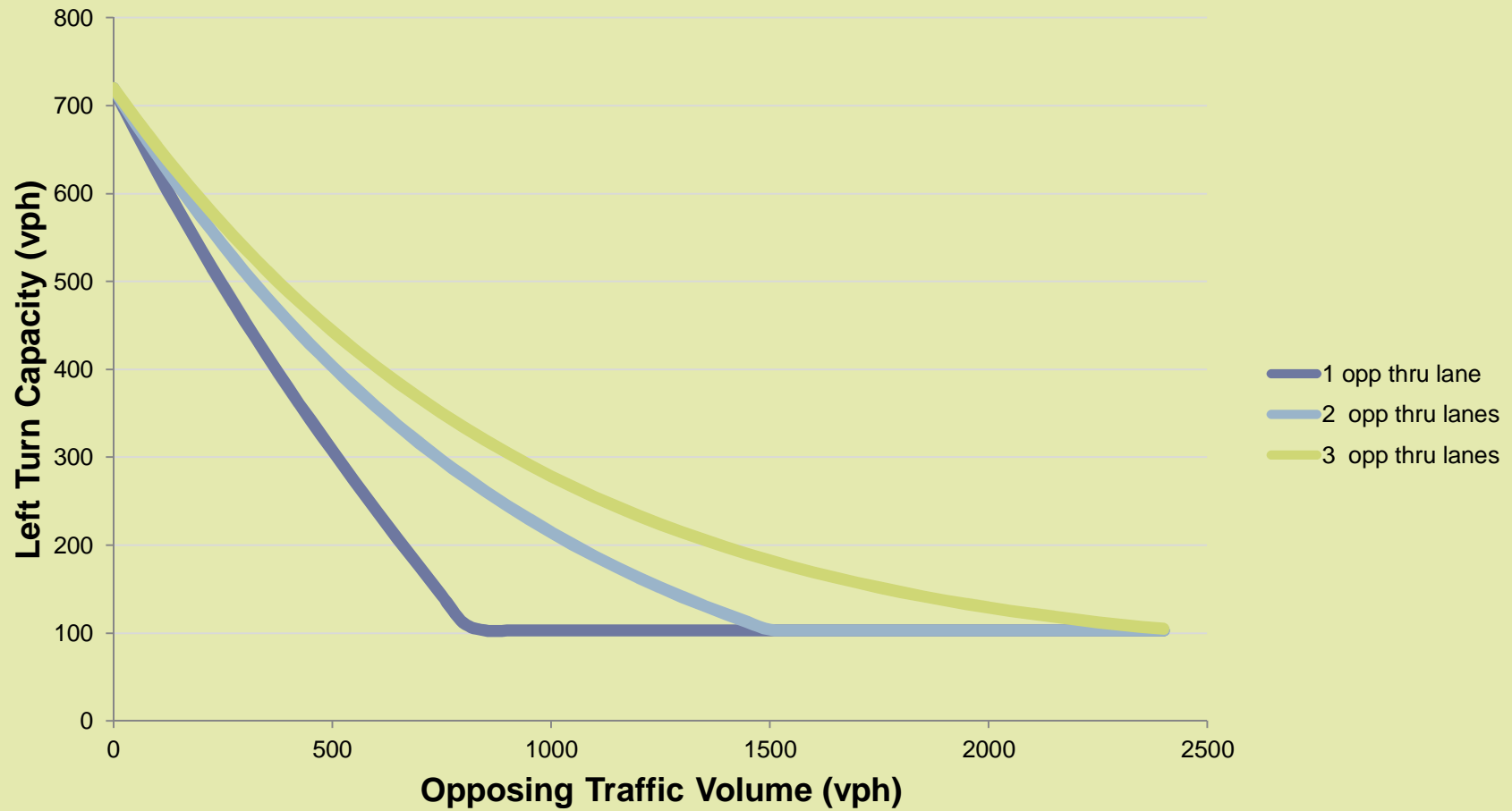


$$g_u = \left(\frac{g_{oT} * 1900 * N_{oT} - C * v_{oT}}{1900 * N_{oT} - v_{oT}} \right) \max .0$$

- g_{oT} = opposing through green time (s)
- N_{oT} = number of opposing through lanes
- v_{oT} = opposing through traffic volume (vph)

Signalised Intersections

Permitted Left Turn Capacity ($g=30$, $C=70$)



Signalised Intersections

- Shared Lanes

- Shared thru and RT lane with no separate dedicated thru lane
- Shared thru and LT lane with no separate dedicated thru lane
- Shared thru and RT lane with parallel dedicated thru lane(s)
- Shared thru and LT lane with parallel dedicated thru lane(s)
- Shared thru and LT lane, shared thru and RT lane and at least one dedicated thru lane.
- Shared thru and RT lane and a shared thru and LT lane
- Single approach lane for the thru movement and both turns.

Signalised Intersections – Case 1 & 2



- Shared Thru/RT (or LT) lane, no separate thru lane
 - Based on “unshared” capacities and previous assigned volumes

$$c_{SH} = \frac{v_R + v_T}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{IT}}}$$

- v_R = right turn volume (vph) from previous assignment
- v_T = through volume (vph) from previous assignment
- c_{IR} = initial right turn capacity (vph) calculated for dedicated lane
- c_{IT} = initial through capacity (vph) calculated for dedicated lane

Signalised Intersections



- Shared Thru/RT (or LT) lane, no separate thru lane

$$C_R = C_{SH} \frac{v_R}{v_R + v_T} = \frac{v_R}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{IT}}}$$

$$C_T = C_{SH} \frac{v_T}{v_R + v_T} = \frac{v_T}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{IT}}}$$

$$\frac{v_R}{C_R} = \frac{v_T}{C_T} = \frac{v_R + v_T}{C_R + C_T}$$

Signalised Intersections – Case 3 &4

- Shared thru/RT (or LT) lane, with separate thru lane
 - “Initial” capacities (c_{IR} and c_{IT}) assuming dedicated turn lane
 - If
$$\frac{v_T}{c_{IT}} \leq \frac{v_R}{c_{IR}}$$
 - Through vehicles will not use the shared lane
 - Hence: $c_R = c_{IR}$ and $c_T = c_{IT}$

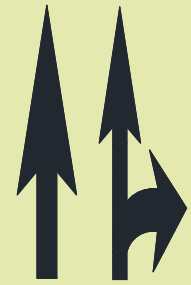


Signalised Intersections

- Shared thru/RT (or LT) lane, with separate thru lane

– If
$$\frac{v_T}{c_{IT}} > \frac{v_R}{c_{IR}}$$

- Some through vehicles will use the shared lane
- And v/c ratios for all lanes will be equal
- Procedure utilises notional capacity c_{ET} for through capacity of shared lane.



Signalised Intersections



- Shared thru/RT (or LT) lane, with separate thru lane

$$c_R = \frac{v_R \left(1 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}}} \quad c_T = \frac{v_T \left(1 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}}}$$

- c_{ET} = additional capacity (vph) if shared lane was a dedicated through lane
- c_R = total right turn capacity (vph) for next assignment cycle
- c_T = total through capacity (vph) for next assignment cycle

$$\frac{v_R}{c_R} = \frac{v_T}{c_T} = \frac{v_R + v_T}{c_R + c_T}$$

Signalised Intersections – Case 5

- Thru/LT lane, thru/RT lane, 1+ thru lane
 - “Initial” capacities (c_{IR} , c_{IL} and c_{IT}) assuming dedicated turn lanes

– If

$$\frac{v_L}{c_{IL}} \geq \frac{v_T}{c_{IT}} \leq \frac{v_R}{c_{IR}}$$

- Through vehicles will not use either shared lane
- Hence: $c_R = c_{IR}$; $c_L = c_{IL}$ and $c_T = c_{IT}$



Signalised Intersections

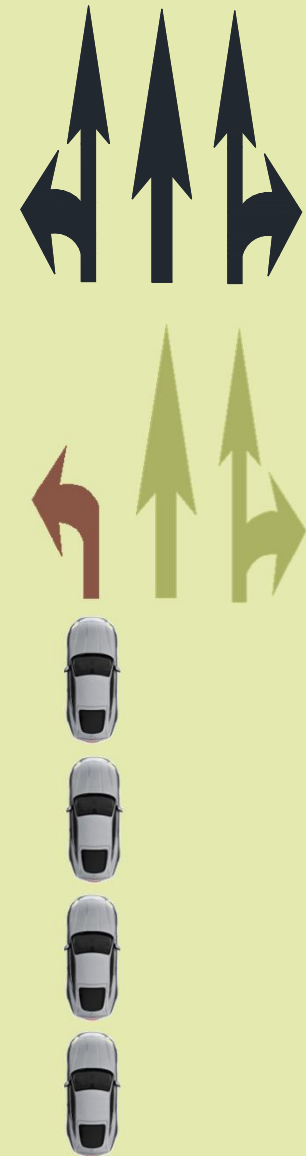
- Thru/LT lane, thru/RT lane, 1+ thru lane

– If
$$\frac{v_L}{c_{IL}} \geq \frac{v_T}{c_{IT}} > \frac{v_R}{c_{IR}}$$

- Thru vehicles will only use dedicated thru lane and shared thru/RT lane
- $c_L = c_{IL}$; c_R and c_T calculated using shared lane procedure

$$c_R = \frac{v_R \left(1 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}}}$$

$$c_T = \frac{v_T \left(1 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}}}$$



Signalised Intersections

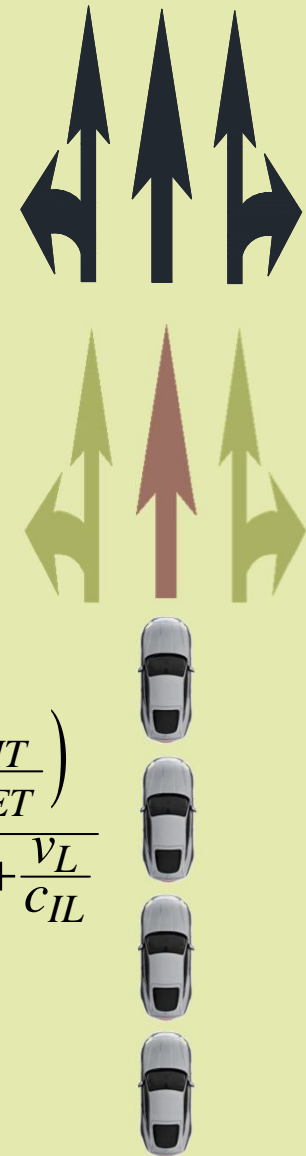
- Thru/LT lane, thru/RT lane, 1+ thru lane

– Finally, if $\frac{v_L}{c_{IL}} < \frac{v_T}{c_{IT}} > \frac{v_R}{c_{IR}}$

- Thru vehicles will use both shared lanes
- c_L ; c_R and c_T calculated using shared lane procedure

$$c_R = \frac{v_R \left(2 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}} \quad c_L = \frac{v_L \left(2 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}} \quad c_T = \frac{v_T \left(2 + \frac{c_{IT}}{c_{ET}}\right)}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}}$$

$$\frac{v_R}{c_R} = \frac{v_T}{c_T} = \frac{v_L}{c_L} = \frac{v_R + v_T + v_L}{c_R + c_T + c_L}$$



Signalised Intersections – Case 6

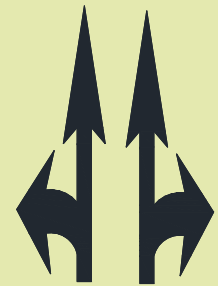
- Thru/LT lane, thru/RT lane only
 - If either but not both lanes overloaded

$$\frac{v_L}{c_{IL}} > 1 \quad \text{or} \quad \frac{v_R}{c_{IR}} > 1$$

- Overloaded lane treated as dedicated
- Remaining lane shared calculation follows case 1 (or 2)
- In all other cases, thru vehicles will use both shared lanes

$$c_R = \frac{2v_R}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}} \quad c_L = \frac{2v_L}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}} \quad c_T = \frac{2v_T}{\frac{v_R}{c_{IR}} + \frac{v_T}{c_{ET}} + \frac{v_L}{c_{IL}}}$$

$$\frac{v_R}{c_R} = \frac{v_T}{c_T} = \frac{v_L}{c_L} = \frac{v_R + v_T + v_L}{c_R + c_T + c_L}$$



Signalised Intersections – Case 7



- Single lane approach
 - Left turn can block through and RT movements
 - If LT volume < 2 vehicles per cycle
 - Thru and RT will “squeeze” by waiting LT vehicles
 - Approach treated as LT lane and partially blocked shared thru/RT lane

Signalised Intersections



- Single lane approach
 - If LT volume > 2 vehicles per cycle
 - LT vehicles will block other vehicles
 - Full Shared Lane Procedure will apply

$$C_R = \frac{v_R}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{ET}} + \frac{v_L}{C_{IL}}} \quad C_L = \frac{v_L}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{ET}} + \frac{v_L}{C_{IL}}} \quad C_T = \frac{v_T}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{ET}} + \frac{v_L}{C_{IL}}}$$

$$\frac{v_R}{C_R} = \frac{v_T}{C_T} = \frac{v_L}{C_L} = \frac{v_R + v_T + v_L}{C_R + C_T + C_L}$$

Unsignalised Intersections

AECOM Approach

- Unsignalised Intersections
 - Two-Way Stop Control (TWSC)
 - Left Turns from Major Streets
 - Minor Streets Traffic Movements
 - All-Way Stops
 - Roundabouts
 - Highway On Ramps

Unsignalised Intersections

- TWSC Intersection

- Left Turns from Major Streets

- Based on HCM 2010 Procedures for “Rank 2” movements
 - For one or two opposing through lanes:

$$c = \frac{v_c e^{-4.1v_c / 3600}}{1 - e^{-2.2v_c / 3600}}$$

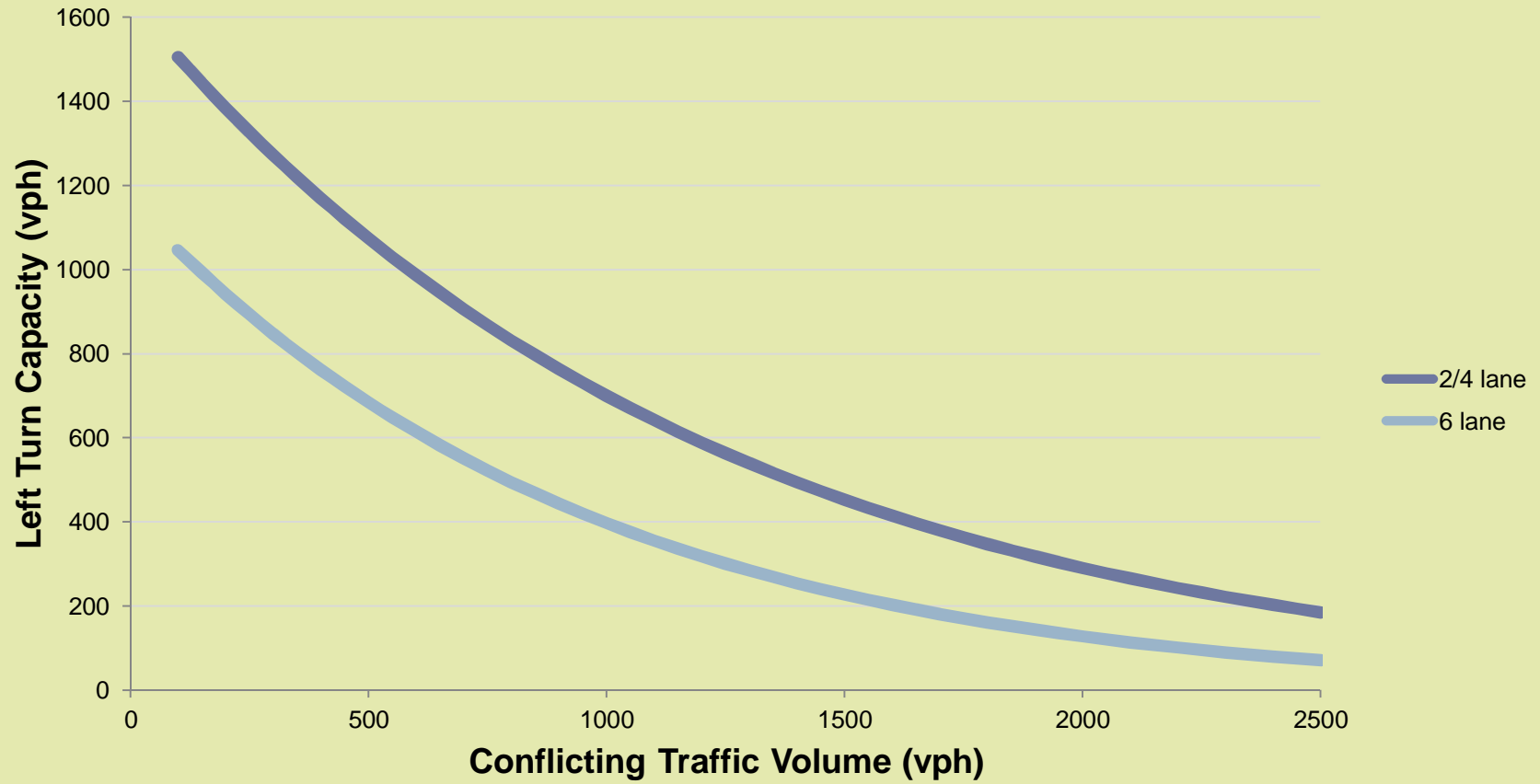
- For more than two opposing through lanes:

$$c = \frac{v_c e^{-5.3v_c / 3600}}{1 - e^{-3.1v_c / 3600}}$$

- $v_c = \text{conflicting volume} = v_{OT} + v_{OR} + v_{pedc}$

Unsignalised Intersections

TWSC - Major Left Turn Capacity



Unsignalised Intersections

- TWSC Intersection

- Minor Street Capacities

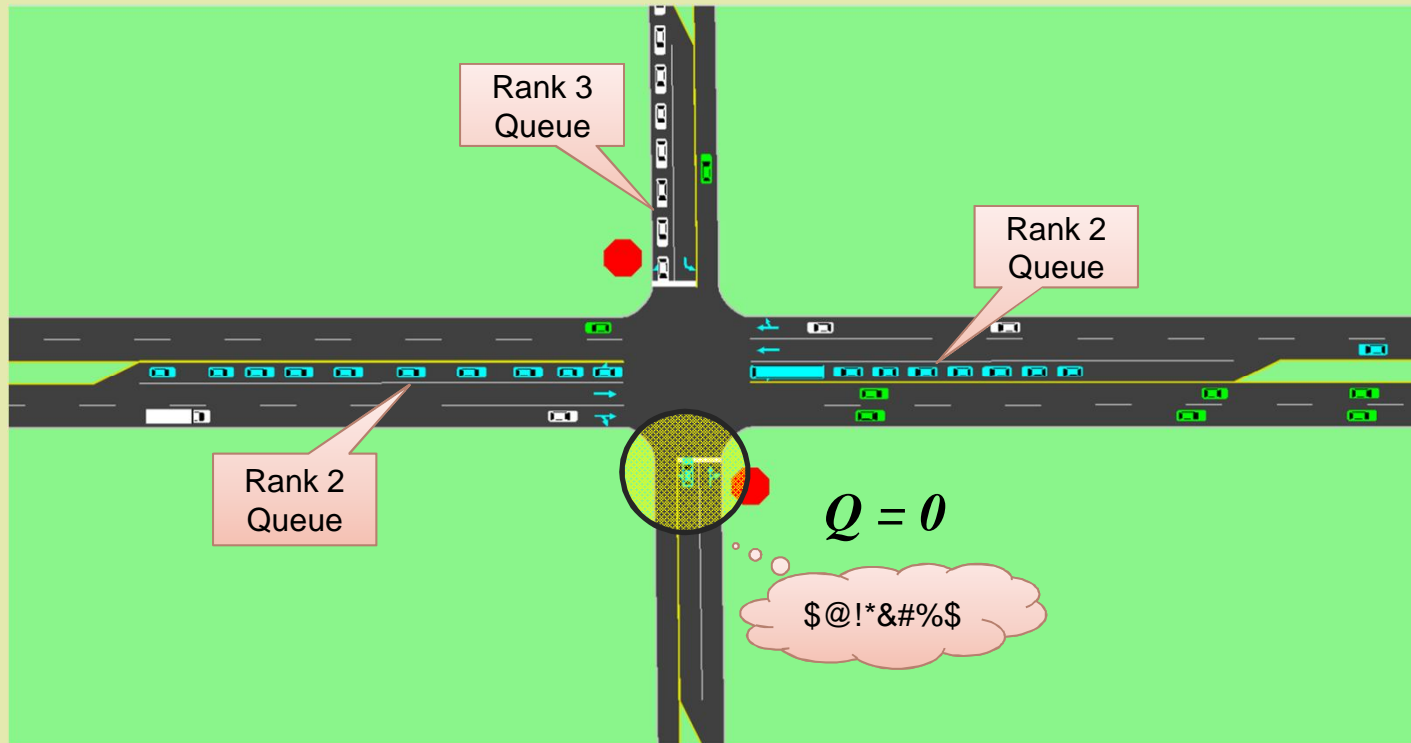
- Simplified HCM 2010 Procedures for “Rank 3” and “Rank 4” movements
 - First, separate capacities calculated for RT, thru movements & LT

$$c_{Ij} = \frac{v_c e^{-6.7 v_c / 3600}}{1 - e^{-3.7 v_c / 3600}} * Q$$

- c_{Ij} = Initial capacity for movement j
 - v_c = Conflicting volume from previous assignment
 - Q = Probability that higher ranked movements are queue-free

Unsignalised Intersections

- TWSC Intersection
 - Minor Street Capacities
 - Q = Probability that higher ranked movements are queue-free



Unsignalised Intersections

- TWSC Intersection

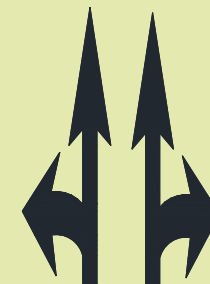
- Minor Street Capacities

- Shared Single Lane

$$C_{SH} = \frac{v_R + v_T + v_L}{\frac{v_R}{C_{IR}} + \frac{v_T}{C_{IT}} + \frac{v_L}{C_{IL}}}$$



- C_{SH} = Minor Road Approach Capacity (vph) saved as link attribute
- Two-Lane Approach
- Shared Lane Procedure for Signalised Intersection (Case 6) followed.



Unsignalised Intersections

- All-Way Stops (& Mini-Roundabouts)
 - Simplified HCM 2010 Procedure
 - Departure Headway assumed 3.2 seconds
 - Basic capacity = 550 vphpl per leg
 - Adjustments for left turns and for unused conflicting capacity
 - Capacities saved as Link Attribute

Unsignalised Intersections

- Roundabouts

- HCM 2010 Procedure

- Capacity per entry lane conflicted by one circulating lane:

$$c = 1130e^{-0.001v_c}$$

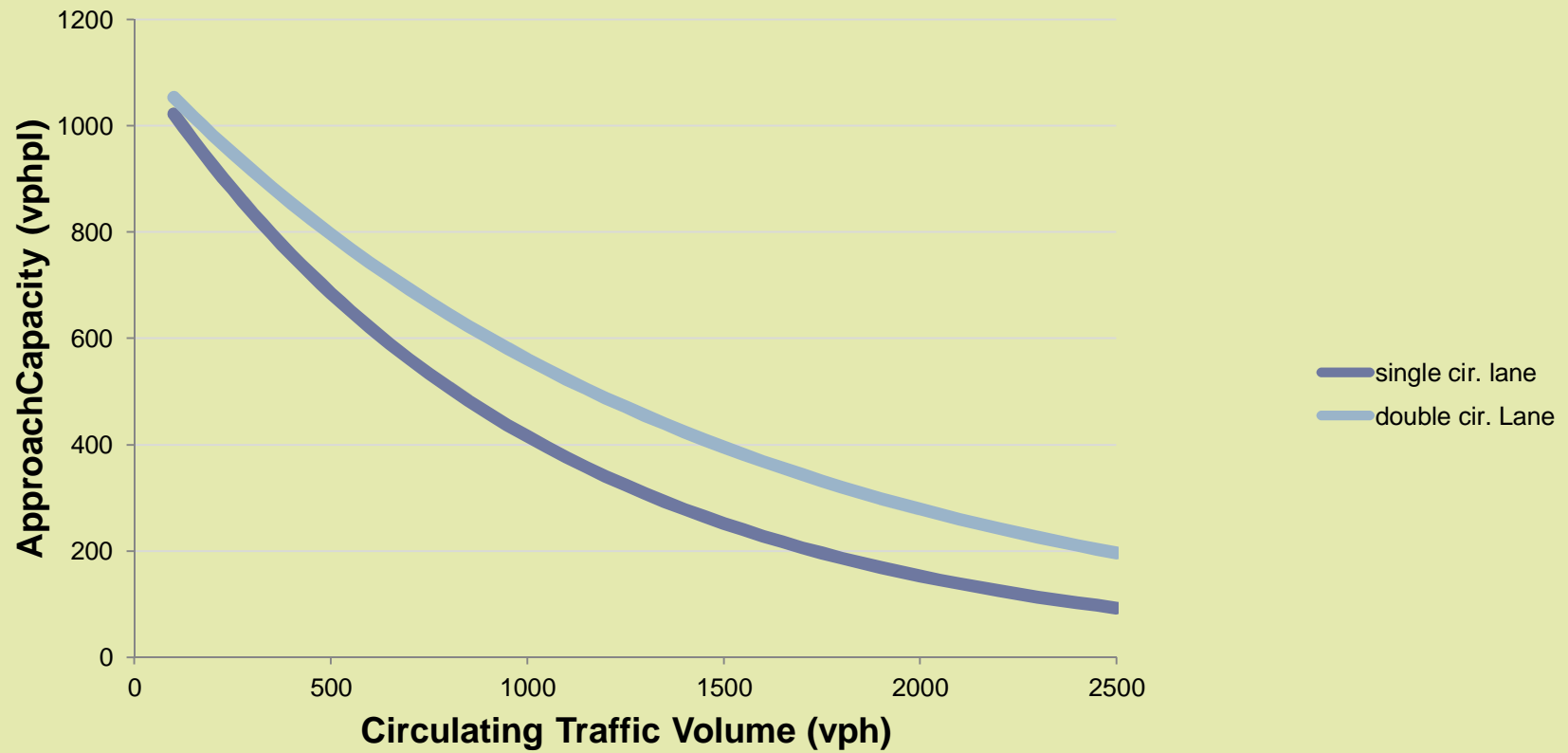
- Capacity per entry lane conflicted by two circulating lanes:

$$c = 1130e^{-0.0007v_c}$$

- Capacities saved as Link Attribute

Unsignalised Intersections

Roundabout Capacity



Unsignalised Intersections

- Highway On Ramps

- For ramp traffic merging with through traffic:

$$C_{Ramp} = c_T * N_T - v_T$$

- For ramp traffic yielding to through traffic:

$$C_{Ramp} = c_T - \frac{v_T}{N_T}$$

- c_T = Capacity of through highway lanes (assumed at 2000 vphpl)
 - N_T = Number of through highway lanes
 - v_T = Upstream through highway volume (vph) from previous assignment

- Capacities saved as Link Attribute

Delay Calculations

- Turn Penalty Functions for intersection delay
- Volume Delay Functions (for 4-way stops, some 2-way stop movements, ramps and merges)
- Uniform Delay
- Congestion-Based Incremental Delay

Delay Calculations

- Uniform (Fixed) Delay (d_1)
 - For protected movements at signalised intersections:

$$d_1 = \frac{(C-g)^2}{2C}$$

- Additional Fixed Delay for permitted (left turn) phases
- Left turns from Major to Minor – $d_1 = 0.05$ minutes
- Minor at 2-way stop – $d_1 = 0.25$ minutes
- 4-way stop and mini-roundabout – $d_1 = 0.2$ minutes
- Other Roundabouts – $d_1 = 0.15$ minutes
- Merge, yield and some right turns have no fixed delay

Delay Calculations

- Variable (Congestion-based) Delay (d_2)
 - BPR-based formula for all intersections

$$d_2 = \left(\frac{v}{c} \right)^4$$

- Conical-based formula is under investigation

Implementation

Implementation

- Capacities calculated using EMME macro – adjcap.mac
- Turn and link attributes used to calculate and store capacities
- Fixed attributes input as part of network coding
- Adjustable attributes calculated in adjcap.mac

Input (Fixed) Attributes

- @type — intersection control type identifier
- @lanes — number of lanes & indication of lane sharing
- @green — green time
- @cycle — cycle length
- @nema — approach direction code and type of phase
- @pedvo — sum of conflicting pedestrian volumes

Calculated (Variable) Attributes

- @assvo — assigned turn movement volumes from previous assignment
- @convo — sum of conflicting or higher ranked volumes from previous assignment
- @sfact — pedestrian-bicycle adjustment factor for right turns and left turns from one-way streets
- @tncap — turn capacity
- @lkcap — link capacity (for 4-way stops, some 2-way stop movements, ramps and merges)

Temporary Attributes

- ul1, ul2, ul3, up1, up2, up3 – used & reused in adjcap.mac
- el1 – Used in vdf for assignment as @lkcap
- ep1 – Used in tpf for assignment as @cycle
- ep2 – Used in tpf for assignment as @green
- ep3 – Used in tpf for assignment as @tncap

Validation Results

Prince George Transportation Study (Earlier Version)

- 100 Intersections Modelled
- Mean absolute error = 18 vehs/hr
- Mean absolute percentage error = 7.4%
- Mean GEH = 1.5
- Max GEH = 9
- $R^2 = 0.986$

Latest Version Being Applied in Study in Surrey

Thank You

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September 2011

